

GLOBAL JOURNAL OF RESEARCHES IN ENGINEERING: E CIVIL AND STRUCTURAL ENGINEERING Volume 23 Issue 3 Version 1.0 Year 2023 Type: Double Blind Peer Reviewed International Research Journal Publisher: Global Journals Online ISSN: 2249-4596 & Print ISSN: 0975-5861

Effect of Seismic Load on Column Forces in RC Structures by Response Spectrum Analysis

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This paper aims to study the effect of seismic load on column forces in different type of RC shear walls in concrete frame structures under different type of soil condition and different load combination. Estimation of column forces such as; column axial force, column moment, column shear force, column torsion, time period and frequency and modal load participation ratios is carried out. In dynamic analysis; Response Spectrum method is used. It was found that the axial force and moment in the column increases when the type of soil changes from hard to medium and medium to soft. Since the column moment increase as the soil type changes, soil structure interaction must be suitably considered while designing frames for seismic force.

Keywords: seismic load, linear dynamics analysis, column forces, high seismic zone. GJRE-E Classification: FOR Code: 0905

EFFECTOFSEISMICLOADONCOLUMN FORCESINRCSTRUCTURES BYRESPONSESPECTRUMANALYSIS

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Keywords: seismic load, linear dynamics analysis, column forces, high seismic zone.

I. INTRODUCTION

a) Structural Systems

n the earliest structures at the beginning of the 20th century, structural members were assumed to carry primarily the gravity loads. Today, however, by the advances in structural design/systems and highstrength materials, building height is increased, which necessitates taking into consideration mainly the lateral loads such as wind and earthquake. Understandably, especially for the tall buildings, as the slenderness, and so the flexibility increases, buildings suffer from the lateral loads resulting from wind and earthquake more and more. As a general rule, when other things being equal, the taller the building, the more necessary it is to identify the proper structural system for resisting the lateral loads. Currently, there are many structural systems that can be used for the lateral resistance of tall buildings[2,3].

Structural systems of tall buildings can be divided into two broad categories: interior structures and exterior structures.

This classification is based on the distribution of the components of the primary lateral load-resisting system over the building.

b) Shear Wall Structure

Shear Wall–Frame Systems (Dual Systems), The system consists of reinforced concrete frames interacting with reinforced concrete shear walls are adequate for resisting both the vertical and the horizontal loads acting on them.

c) Necessity of Shear Walls

Shear wall system has two distinct advantages over a frame system.

- It provides adequate strength to resist large lateral loads with-out excessive additional cost.
- It provides adequate stiffness to resist lateral displacements to permissible limits, thus reducing risk of non-structural damage.

d) Seismic Load

The seismic weight of building is the sum of seismic weight of all the floors [8]. The seismic weight of each floor is its full dead load plus appropriate amount of imposed load, the latter being that part of the imposed loads that may reasonably be expected to be attached to the structure at the time of earthquake shaking. Earthquake forces experienced by a building result from ground motions (accelerations) which are also fluctuating or dynamic in nature, in fact they reverse direction somewhat chaotically[2,3]. In theory and practice, the lateral force that a building experiences from an earthquake increases in direct proportion with the acceleration of ground motion at the building site and the mass of the building. As the ground accelerates back and forth during an earthquake it imparts backand-forth (cyclic) forces to a building through its foundation which is forced to move with the ground [1].

e) Geo-Technical Consideration

The seismic motion that reaches a structure on the surface of the earth is influenced by local soil conditions. The subsurface soil layers underlying the building foundation may amplify the response of the building to earthquake motions originating in the bedrock.

Bearing Capacity of Foundation Soil

Three soil types are considered here:

- I. Hard Those soils, which have an allowable bearing capacity of more than 10t/m2.
- II. Medium Those soils, which have an allowable bearing capacity less than or equal to 10t/m2.

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III. Soft - Those soils, which are liable to large differential settlement or liquefaction during an earthquake.

The allowable bearing pressure shall be determined in accordance with IS: 1888-1982 load test (Revision 1992).

II. Methodology

- a) To understand and evaluation building structures and aims to the effect of Seismic load on column Forces in Different Type of RC Shear Walls in Concrete Frame Structures under Different Type of Soil Condition with seismic loading.
- b) Modeling a G+29 story high building for five different cases [9-11].
- c) Analyzing the building dynamic analysis using linear, i.e. Response Spectrum Analysis [1-3].
- d) Analyzing the results and arriving at conclusions.

a) Dynamic Analysis

Dynamic analysis may be executed to get the design seismic force, and its spread in different levels through the height of the building, and also various lateral load resisting element[1-2-3,8].

b) Response Spectrum Method

This method is executed to design spectrum, where as it is specified with a code for specific- site design can be used for a project site for the purposes of dynamic of steel and reinforce concrete buildings, the values of damping for building may be taken as 2 and 5 percent of the critical, respectively. response spectrum method is typically implemented in linear elastic procedures and also very much easier to use. This also called as or mode superposition method or model method, It also made on the idea of the superposition of responses given by the building through various modes of vibrations, each vibration modes is recorded as with its own particular deformed shape, with its own modal damping and its own frequency [7,8].

III. MODELING OF BUILDING

a) Details of the Building

A symmetrical building[15] of plan 38.5m X 35.5m located with location in high Seismic zone considered. Four bays of length 7.5m & one bays of length 8.5m along X - direction and four bays of length 7.5m & one bays of length 5.5m along Y - direction are provided. Shear is provided the center inner core of model building.

Struct I: G+29 story'stall building with Plus shape RC shear wall at the center of structure.

Struct II: G+29 story'stall building with Box shape RC shear wall at the center of structure.

Struct III: G+29 story'stall building with C- shape RC shear wall at the center of structure.

Struct IV: G+29 story'stall building with E- shape RC shear wall at the center of structure.

Struct V: G+29 story'stall building with I- shape RC shear wall at the center of structure.

b) Load Combinations

As per IS 1893 (Part 1): 2002 Clause no. 6.3.1.2, the following load cases have to be considered for analysis:

"1.2 (DL + IL ± EL)"

"1.5 (DL ± EL)"

"EQXP&EQYP"

Earthquake load must be considered for +X, -X, +Y and -Y Directions [5-7].

c) The Building Details

Type of frame: Special RC moment resisting frame fixed at the base, Number of storeys: G+29, Floor height: 3.5 m, Depth of Slab: 225 mm, Size of beam: (300×600) mm, Size of column (exterior): (1250×1250) mm up to story five, Size of column (exterior): (900×900) mm Above story five, Size of column (interior): (1250×1250) mm up to story ten, Size of column (interior): (900×900) mm Above story ten, Live load on floor: 4 KN/m2, Floor finish: 2.5 KN/m2, Wall load: 25 KN/m, Grade of Concrete: M 50 concrete, Grade of Steel: Fe 500, Thickness of shear wall: 450 mm, Seismic zone: V, Important Factor: 1.5, Density of concrete: 25 KN/m3, Type of soil: Type I=Soft Soil, Type II=Medium Soil, Type III = Hard Soil, Response spectra: As per IS 1893(Part-1):2002, Damping of structure: 5 percent & All the analyses has been carried out as per the Indian Standard code books [4-8].



Figure 2: 3D view showing shear wall location for Structure I



Figure 3: Plan of the Structure II



Figure 4: 3D view showing shear wall location for Structure II







Figure 6: 3D view showing shear wall location for Structure III



Figure 7: Plan of the Structure IV



Figure 8: 3D view showing shear wall location for Structure IV



Figure 10: 3D view showing shear wall location for Structure V

IV. Results and Discussions

Parametric results in column forces such as column axial force, column moment, column shear force & column torsion with different load combination/load Cases such as 1.2 (DL+LL+EQXP), 1.2 (DL+LL+EQXP), 1.5 (DL+EQXP), 1.5 (DL+EQYP),

EQXP & EQYP in different type of soil conditions (soft, medium and hard) were considered, in this regard we compared all column forces in different type of soil condition of structures II, III, IV, V with structure I (plus shape shear wall), also compared forces in hard and medium soils with soft soil for all five structures.

Table 1: Column Axial Force, P for structures with the load combination 1.2 (DL+LL+EQXP) & 1.2 (DL+LL+EQYP), All value in "kN"

	Col	umn Axial F	orce, P in Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V
Story	"Column"	"Unique - Name"	"Load Case- Combo"	"Station"m	"P"	"P"	"P"	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0	-24171.0618	-24285.0493	-24629.8602	-24381.5444	-24398.1773
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-24103.093	-24217.0806	-24561.8915	-24313.5757	-24330.2086
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-24035.1243	-24149.1118	-24493.9227	-24245.6069	-24262.2398
1ST	C34	67	1.2(DL+LL+EQYP)	0	-23630.6382	-23276.1711	-23447.6424	-23345.1752	-23441.1649
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	-23562.6694	-23208.2023	-23379.6736	-23277.2065	-23373.1961
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	-23494.7007	-23140.2336	-23311.7049	-23209.2377	-23305.2274
			Co	umn Axial Ford	ce, P in Medium	n Soil			
1ST	C34	67	1.2(DL+LL+EQXP)	0	-24937.4993	-25121.0698	-25571.6279	-25446.3503	-25240.6514
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-24869.5305	-25053.1011	-25503.6591	-25378.3816	-25172.6826
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-24801.5618	-24985.1323	-25435.6904	-25310.4128	-25104.7139
1ST	C34	67	1.2(DL+LL+EQYP)	0	-24202.5232	-23748.9954	-23963.8116	-23949.6572	-23939.1144
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	-24134.5545	-23681.0267	-23895.8428	-23881.6884	-23871.1456
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	-24066.5857	-23613.0579	-23827.8741	-23813.7197	-23803.1769
			С	olumn Axial Fo	rce, P in Hard S	Soil			
1ST	C34	67	1.2(DL+LL+EQXP)	0	-25597.4871	-25840.9764	-26382.5944	-26235.5482	-25966.1151
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-25529.5184	-25773.0076	-26314.6257	-26167.5794	-25898.1464
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-25461.5496	-25705.0389	-26246.6569	-26099.6107	-25830.1776
1ST	C34	67	1.2(DL+LL+EQYP)	0	-24694.9798	-24156.1497	-24408.2906	-24397.697	-24367.9043
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	-24627.011	-24088.181	-24340.3219	-24329.7283	-24299.9355
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	-24559.0423	-24020.2122	-24272.3531	-24261.7595	-24231.9668

	Colum	n Mome	ent, M in Sof	: Soil	Struct	Struct	Struct	Struct	Struct	Struct	Struct IV	Struct IV	Struct V	Struct V
Sto ry	Colu mn	Uniq ue Name	Load Case/Com bo	Stati on m	"M2"	"M3"	"M2"	"M3"	"M2"	"МЗ"	"M2"	"M3"	"M2"	"M3"
1S T	C34	67	1.2(DL+LL+ EQXP)	0	- 244.01 18	979.47 15	- 171.67 74	1061.1 112	- 251.86 41	1421.2 435	- 239.99 22	1271.7 973	- 249.77 58	971.72 83
1S T	C34	67	1.2(DL+LL+ EQXP)	1.45	- 146.26 84	805.69 93	- 84.416 8	912.71 96	- 151.39 27	1219.8 181	- 142.18 6	1095.4 925	- 150.87 48	826.99 06
1S T	C34	67	1.2(DL+LL+ EQXP)	2.9	- 48.525 1	631.92 71	2.8438	764.32 8	- 50.921 3	1018.3 927	- 44.379 9	919.18 78	- 51.973 8	682.25 29
1S T	C34	67	1.2(DL+LL+ EQYP)	0	1727.5 733	- 24.707 5	1026.4 07	- 134.63 53	1218.6 199	- 173.18 54	1153.6 344	- 157.40 43	1174.9 664	- 74.852 3
1S T	C34	67	1.2(DL+LL+ EQYP)	1.45	1393.6 416	- 70.519 4	893.97 23	- 94.628	1027.4 053	- 112.27 58	974.88 51	- 107.00 72	954.74 75	- 81.408 3
1S T	C34	67	1.2(DL+LL+ EQYP)	2.9	1059.7 1	- 116.33 13	761.53 75	- 54.620 7	836.19 07	- 51.366 3	796.13 58	- 56.610 1	734.52 87	- 87.964 4
					Col	umn Mor	ment, M i	n Mediun	n Soil					
1S T	C34	67	1.2(DL+LL+ EQXP)	0	- 312.52 42	1329.5 266	- 216.79	1461.8 423	- 325.85 38	1958.0 803	- 325.92 7	1862.7 469	- 322.56 99	1328.7 543
1S T	C34	67	1.2(DL+LL+ EQXP)	1.45	- 197.67 08	1112.7 719	- 115.99 39	1264.1 942	- 207.08 2	1683.6 228	- 206.75 27	1610.8 77	- 205.97 96	1142.9 081
1S T	C34	67	1.2(DL+LL+ EQXP)	2.9	- 82.817 5	896.01 72	- 15.197 8	1066.5 461	- 88.310 2	1409.1 652	- 87.578 5	1359.0 072	- 89.389 3	957.06 19
1S T	C34	67	1.2(DL+LL+ EQYP)	0	2368.8 316	- 36.156 8	1412.6 049	- 164.37 29	1674.0 045	- 210.34 29	1686.2 828	- 200.78 17	1615.0 795	- 94.595 2
1S T	C34	67	1.2(DL+LL+ EQYP)	1.45	1896.6 069	- 78.885 5	1214.6 153	- 105.79 85	1396.0 833	- 128.02 5	1406.1 652	- 125.34 18	1297.6 668	- 92.514 4
1S T	C34	67	1.2(DL+LL+ EQYP)	2.9	1424.3 822	- 121.61 42	1016.6 256	- 47.224 2	1118.1 621	- 45.707	1126.0 477	- 49.901 9	980.25 41	- 90.433 6
	1	1	r	1	С	olumn M	oment, M	in Hard	Soil					
1S T	C34	67	1.2(DL+LL+ EQXP)	0	- 371.52 09	1630.9 629	- 255.63 69	1806.9 164	- 389.56 71	2420.3 565	- 389.65 26	2300.9 465	- 385.25 37	1636.1 935
1S T	C34	67	1.2(DL+LL+ EQXP)	1.45	- 241.93 4	1377.1 956	- 143.18 53	1566.8 529	- 255.03 67	2083.0 102	- 254.63 2	1993.0 377	- 253.43 1	1414.9 482
1S T	C34	67	1.2(DL+LL+ EQXP)	2.9	- 112.34 71	1123.4 282	- 30.733 6	1326.7 894	- 120.50 62	1745.6 638	- 119.61 13	1685.1 289	- 121.60 82	1193.7 03
1S T	C34	67	1.2(DL+LL+ EQYP)	0	2921.0 262	- 46.015 9	1745.1 642	- 189.98 02	2066.1 412	- 242.33 97	2081.2 226	- 232.94 53	1994.0 659	- 111.59 61
1S T	C34	67	1.2(DL+LL+ EQYP)	1.45	2329.7 158	- 86.089 7	1490.7 245	- 115.41 76	1713.5 56	- 141.58 67	1725.9 364	- 138.93 69	1592.9 584	- 102.07 8
1S T	C34	67	1.2(DL+LL+ EQYP)	2.9	1738.4 055	- 126.16 34	1236.2 848	- 40.855	1360.9 708	- 40.833 8	1370.6 502	- 44.928 5	1191.8 51	- 92.559 9

Table 2: Column Moment, M for structures with the load combination 1.2 (DL+LL+EQXP) &1.2 (DL+LL+EQYP), All value in "kN-m"

	Colu	mn She	ear, V in Soft So	oil	Struct	Struct	Struct	Struct	Struct	Struct	Struct IV	Struct IV	Struct V	Struct V
Sto ry	Colu mn	Uniq ue Name	Load Case/Combo	Stati on m	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"	"V2"	"∨3"
1ST	C34	67	1.2(DL+LL+ EQXP)	0	119.8 429	- 67.40 92	102.3 39	- 60.17 98	138.9 141	- 69.29 06	121.5 895	- 67.45 25	99.81 91	- 68.20 76
1ST	C34	67	1.2(DL+LL+ EQXP)	1.45	119.8 429	- 67.40 92	102.3 39	- 60.17 98	138.9 141	- 69.29 06	121.5 895	- 67.45 25	99.81 91	- 68.20 76
1ST	C34	67	1.2(DL+LL+ EQXP)	2.9	119.8 429	- 67.40 92	102.3 39	- 60.17 98	138.9 141	- 69.29 06	121.5 895	- 67.45 25	99.81 91	- 68.20 76
1ST	C34	67	1.2(DL+LL+ EQYP)	0	31.59 44	230.2 977	- 27.59 12	91.33 43	- 42.00 66	131.8 722	- 34.75 66	123.2 754	4.521 4	151.8 751
1ST	C34	67	1.2(DL+LL+ EQYP)	1.45	31.59 44	230.2 977	- 27.59 12	91.33 43	- 42.00 66	131.8 722	- 34.75 66	123.2 754	4.521 4	151.8 751
1ST	C34	67	1.2(DL+LL+ EQYP)	2.9	31.59 44	230.2 977	- 27.59 12	91.33 43	- 42.00 66	131.8 722	- 34.75 66	123.2 754	4.521 4	151.8 751
					Colu	ımn Shea	ar, V in M	edium S	oil					
1ST	C34	67	1.2(DL+LL+ EQXP)	0	149.4 86	- 79.20 92	136.3 091	- 69.51 45	189.2 811	- 81.91 16	173.7 034	- 82.18 92	128.1 698	- 80.40 71
1ST	C34	67	1.2(DL+LL+ EQXP)	1.45	149.4 86	- 79.20 92	136.3 091	- 69.51 45	189.2 811	- 81.91 16	173.7 034	- 82.18 92	128.1 698	- 80.40 71
1ST	C34	67	1.2(DL+LL+ EQXP)	2.9	149.4 86	- 79.20 92	136.3 091	- 69.51 45	189.2 811	- 81.91 16	173.7 034	- 82.18 92	128.1 698	- 80.40 71
1ST	C34	67	1.2(DL+LL+ EQYP)	0	29.46 81	325.6 722	- 40.39 61	136.5 446	- 56.77 1	191.6 698	- 52.02 75	193.1 845	-1.435	218.9 053
1ST	C34	67	1.2(DL+LL+ EQYP)	1.45	29.46 81	325.6 722	- 40.39 61	136.5 446	- 56.77 1	191.6 698	- 52.02 75	193.1 845	-1.435	218.9 053
1ST	C34	67	1.2(DL+LL+ EQYP)	2.9	29.46 81	325.6 722	- 40.39 61	136.5 446	- 56.77 1	191.6 698	- 52.02 75	193.1 845	-1.435	218.9 053
					Co	lumn Sh	ear, V in	Hard Soi						
1ST	C34	67	1.2(DL+LL+ EQXP)	0	175.0 12	- 89.37 03	165.5 61	- 77.55 28	232.6 527	- 92.77 96	212.3 509	- 93.11 77	152.5 829	- 90.91 22
1ST	C34	67	1.2(DL+LL+ EQXP)	1.45	175.0 12	- 89.37 03	165.5 61	- 77.55 28	232.6 527	- 92.77 96	212.3 509	- 93.11 77	152.5 829	- 90.91 22
1ST	C34	67	1.2(DL+LL+ EQXP)	2.9	175.0 12	- 89.37 03	165.5 61	- 77.55 28	232.6 527	- 92.77 96	212.3 509	- 93.11 77	152.5 829	- 90.91 22
1ST	C34	67	1.2(DL+LL+ EQYP)	0	27.63 71	407.8 002	- 51.42 25	175.4 757	- 69.48 48	243.1 622	- 64.83 34	245.0 25	- 6.564 2	276.6 258
1ST	C34	67	1.2(DL+LL+ EQYP)	1.45	27.63 71	407.8 002	- 51.42 25	175.4 757	- 69.48 48	243.1 622	- 64.83 34	245.0 25	- 6.564 2	276.6 258
1ST	C34	67	1.2(DL+LL+ EQYP)	2.9	27.63 71	407.8 002	- 51.42	175.4 757	- 69.48	243.1 622	- 64.83 34	245.0 25	- 6.564 2	276.6 258

Table 3: Column Shear, V for structures with the load combination 1.2 (DL+LL+EQXP) & 1.2 (DL+LL+EQYP), All value in "KN"

Table 4: Column Torsion, T for structures with the load combination 1.2 (DL+LL+EQXP) & 1.2 (DL+LL+EQYP), All value in "kN-m"

		Column Torsi	on, T in Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V
Story	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"	"T"	"T"	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0	-41.6175	-29.3334	-44.901	-42.3525	-43.8436
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-41.6175	-29.3334	-44.901	-42.3525	-43.8436
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-41.6175	-29.3334	-44.901	-42.3525	-43.8436
1ST	C34	67	1.2(DL+LL+EQYP)	0	45.3145	31.9525	48.8724	46.1375	48.5638
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	45.3145	31.9525	48.8724	46.1375	48.5638
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	45.3145	31.9525	48.8724	46.1375	48.5638
			•	Column Torsion,	T in Medium Sc	bil			
1ST	C34	67	1.2(DL+LL+EQXP)	0	-56.5981	-39.8539	-61.0208	-61.1008	-59.584
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-56.5981	-39.8539	-61.0208	-61.1008	-59.584
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-56.5981	-39.8539	-61.0208	-61.1008	-59.584
1ST	C34	67	1.2(DL+LL+EQYP)	0	61.6294	43.4949	66.5111	66.66	66.09
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	61.6294	43.4949	66.5111	66.66	66.09
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	61.6294	43.4949	66.5111	66.66	66.09
			•	Column Torsio	n, T in Hard Soil				
1ST	C34	67	1.2(DL+LL+EQXP)	0	-69.4981	-48.9132	-74.9017	-75.004	-73.1383
1ST	C34	67	1.2(DL+LL+EQXP)	1.45	-69.4981	-48.9132	-74.9017	-75.004	-73.1383
1ST	C34	67	1.2(DL+LL+EQXP)	2.9	-69.4981	-48.9132	-74.9017	-75.004	-73.1383
1ST	C34	67	1.2(DL+LL+EQYP)	0	75.6784	53.4342	81.6999	81.8788	81.182
1ST	C34	67	1.2(DL+LL+EQYP)	1.45	75.6784	53.4342	81.6999	81.8788	81.182
1ST	C34	67	1.2(DL+LL+EQYP)	2.9	75.6784	53.4342	81.6999	81.8788	81.182

Table 5: Column Axial Force, P for structures with the load combination 1.5 (DL+EQXP) & 1.5 (DL+EQYP), All value in "kN"

	Co	olumn Axial F	Force, Pin Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"	"P"	"P"	"P"
1ST	C34	67	1.5(DL+EQXP)	0	-25183.8699	-25355.396	-25767.3656	-25468.0736	-25450.8356
1ST	C34	67	1.5(DL+EQXP)	1.45	-25098.9089	-25270.435	-25682.4047	-25383.1127	-25365.8747
1ST	C34	67	1.5(DL+EQXP)	2.9	-25013.948	-25185.4741	-25597.4437	-25298.1518	-25280.9137
1ST	C34	67	1.5(DL+EQYP)	0	-24508.3404	-24094.2982	-24289.5933	-24172.6121	-24254.57
1ST	C34	67	1.5(DL+EQYP)	1.45	-24423.3794	-24009.3372	-24204.6324	-24087.6512	-24169.6091
1ST	C34	67	1.5(DL+EQYP)	2.9	-24338.4185	-23924.3763	-24119.6714	-24002.6903	-24084.6481
				Column Axia	l Force, P in Medi	um Soil			
1ST	C34	67	1.5(DL+EQXP)	0	-26141.9168	-26400.4216	-26944.5752	-26799.081	-26503.9282
1ST	C34	67	1.5(DL+EQXP)	1.45	-26056.9558	-26315.4607	-26859.6142	-26714.1201	-26418.9672
1ST	C34	67	1.5(DL+EQXP)	2.9	-25971.9949	-26230.4998	-26774.6533	-26629.1591	-26334.0063
1ST	C34	67	1.5(DL+EQYP)	0	-25223.1967	-24685.3286	-24934.8048	-24928.2146	-24877.0069
1ST	C34	67	1.5(DL+EQYP)	1.45	-25138.2357	-24600.3677	-24849.8439	-24843.2537	-24792.046
1ST	C34	67	1.5(DL+EQYP)	2.9	-25053.2748	-24515.4068	-24764.8829	-24758.2927	-24707.0851
				Column Ax	ial Force, P in Har	d Soil		•	
1ST	C34	67	1.5(DL+EQXP)	0	-26966.9016	-27300.3048	-27958.2834	-27785.5783	-27410.7578
1ST	C34	67	1.5(DL+EQXP)	1.45	-26881.9407	-27215.3439	-27873.3224	-27700.6174	-27325.7969
1ST	C34	67	1.5(DL+EQXP)	2.9	-26796.9797	-27130.383	-27788.3615	-27615.6564	-27240.836
1ST	C34	67	1.5(DL+EQYP)	0	-25838.7674	-25194.2715	-25490.4036	-25488.2644	-25412.9943
1ST	C34	67	1.5(DL+EQYP)	1.45	-25753.8064	-25109.3106	-25405.4426	-25403.3035	-25328.0334
1ST	C34	67	1.5(DL+EQYP)	2.9	-25668.8455	-25024.3496	-25320.4817	-25318.3425	-25243.0724

Table 6: Column Moment, M for structures with the load combination 1.5 (DL + EQXP) & 1.5 (DL+EQYP), A	Il value in
"kN-m"	

	Column	n Moment, N	I in Soft Soil		Struct	Struct V	Struct V							
"Story"	"Column"	"Unique - Name"	"Load Case- Combo"	"Station"m	"M2"	"МЗ"	"M2"	"МЗ"	"M2"	"M3"	"M2"	"M3"	"M2"	"M3"
1ST	C34	67	1.5(DL+ EQXP)	0	- 300.97 13	1225.7 47	- 213.58	1343.4 34	- 313.8 242	1800.2 079	- 298.97 71	1609.5 397	- 311.21 44	1219.4 677
1ST	C34	67	1.5(DL+ EQXP)	1.45	- 185.76 63	1027.6 976	- 111.24 83	1165.8 496	- 194.9 693	1551.5 389	- 183.46 03	1395.0 708	- 194.32 03	1051.1 982
1ST	C34	67	1.5(DL+ EQXP)	2.9	- 70.561 4	829.64 82	- 8.9167	988.26 53	- 76.11 45	1302.8 699	- 67.943 6	1180.6 019	- 77.426 3	882.92 88
1ST	C34	67	1.5(DL+ EQYP)	0	2163.5 101	- 29.476 6	1284.0 256	- 151.24 91	1524. 2808	- 192.82 82	1443.0 562	- 176.96 23	1469.7 135	- 88.758
1ST	C34	67	1.5(DL+ EQYP)	1.45	1739.1 213	- 67.575 7	1111.7 38	- 93.334 9	1278. 5282	- 113.57 85	1212.8 786	- 108.05 39	1187.7 076	- 84.300 4
1ST	C34	67	1.5(DL+ EQYP)	2.9	1314.7 324	- 105.67 48	939.45 04	- 35.420 6	1032. 7756	- 34.328 8	982.70 11	- 39.145 4	905.70 18	- 79.842 9
					Co	umn Mome	ent, M in M	edium Soi	I					
1ST	C34	67	1.5(DL+ EQXP)	0	- 386.6 118	1663.3 159	- 269.97 07	1844. 3479	- 406.31 13	2471.2 54	- 406.39 56	2348.2 268	- 402.20 7	1665.7 503
1ST	C34	67	1.5(DL+ EQXP)	1.45	- 250.0 193	1411.5 384	- 150.71 97	1605. 1929	- 264.58 1	2131.2 948	- 264.16 87	2039.3 015	- 263.20 13	1446.0 951
1ST	C34	67	1.5(DL+ EQXP)	2.9	- 113.4 269	1159.7 609	- 31.468 7	1366. 0378	- 122.85 06	1791.3 356	- 121.94 18	1730.3 762	- 124.19 57	1226.4 4
1ST	C34	67	1.5(DL+ EQYP)	0	2965. 0829	- 43.788 3	1766.7 729	- 188.4 211	2093.5 115	- 239.27 51	2108.8 666	- 231.18 4	2019.8 549	- 113.43 67
1ST	C34	67	1.5(DL+ EQYP)	1.45	2367. 8278	- 78.033 4	1512.5 417	- 107.2 981	1739.3 757	- 133.26 49	1751.9 788	- 130.97 2	1616.3 567	- 98.183
1ST	C34	67	1.5(DL+ EQYP)	2.9	1770. 5727	- 112.27 85	1258.3 105	- 26.17 5	1385.2 398	- 27.254 7	1395.0 909	- 30.760 1	1212.8 586	- 82.929 4
					С	olumn Mor	nent, M in I	Hard Soil						
1ST	C34	67	1.5(DL+ EQXP)	0	- 460.3 577	2040.1 114	- 318.52 93	2275. 6905	- 485.95 3	3049.0 992	- 486.05 26	2895.9 762	- 480.56 17	2050.0 492
1ST	C34	67	1.5(DL+ EQXP)	1.45	- 305.3 483	1742.0 68	- 184.70 89	1983. 5162	- 324.52 43	2630.5 29	- 324.01 77	2517.0 023	- 322.51 55	1786.1 453
1ST	C34	67	1.5(DL+ EQXP)	2.9	- 150.3 389	1444.0 245	- 50.888 5	1691. 342	- 163.09 56	2211.9 588	- 161.98 28	2138.0 283	- 164.46 93	1522.2 413
1ST	C34	67	1.5(DL+ EQYP)	0	3655. 3261	- 56.112 2	2182.4 72	- 220.4 303	2583.6 823	- 279.27 11	2602.5 414	- 271.38 85	2493.5 878	- 134.68 77
1ST	C34	67	1.5(DL+ EQYP)	1.45	2909. 214	- 87.038 6	1857.6 783	- 119.3 219	2136.2 165	- 150.21 71	2151.6 928	- 147.96 59	1985.4 712	- 110.13 75
1ST	C34	67	1.5(DL+ EQYP)	2.9	2163. 1019	- 117.96 5	1532.8 845	- 18.21 35	1688.7 508	- 21.163 2	1700.8 441	- 24.543 4	1477.3 547	- 85.587 3

Table 7: Column Shear, V for structures with the load combination 1.5 (DL+EQXP) & 1.5 (DL+EQYP), All value in "kN"

	Colum	nn Shear, Vi	in Soft Soil		Struct	Struct	Struct	Struct	Struct	Struct	Struct	Struct	Struct V	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case- Combo"	"Station"m	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"
1ST	C34	67	1.5(DL+ EQXP)	0	136.58 58	- 79.45 17	122.47 2	- 70.573 6	171.49 59	- 81.968 9	147.90 96	- 79.666 7	116.04 79	- 80.61 66
1ST	C34	67	1.5(DL+ EQXP)	1.45	136.58 58	- 79.45 17	122.47 2	- 70.573 6	171.49 59	- 81.968 9	147.90 96	- 79.666 7	116.04 79	- 80.61 66
1ST	C34	67	1.5(DL+ EQXP)	2.9	136.58 58	- 79.45 17	122.47 2	- 70.573 6	171.49 59	- 81.968 9	147.90 96	- 79.666 7	116.04 79	- 80.61 66
1ST	C34	67	1.5(DL+ EQYP)	0	26.275 2	292.6 819	- 39.940 9	118.81 9	- 54.654 9	169.48 46	- 47.523 1	158.74 31	- 3.0742	194.4 868
1ST	C34	67	1.5(DL+ EQYP)	1.45	26.275 2	292.6 819	- 39.940 9	118.81 9	- 54.654 9	169.48 46	- 47.523 1	158.74 31	- 3.0742	194.4 868
1ST	C34	67	1.5(DL+ EQYP)	2.9	26.275 2	292.6 819	- 39.940 9	118.81 9	- 54.654 9	169.48 46	- 47.523 1	158.74 31	- 3.0742	194.4 868
					Co	olumn Shea	ar, V in Meo	dium Soil						
1ST	C34	67	1.5(DL+ EQXP)	0	173.63 97	- 94.201 7	164.93 45	- 82.242 1	234.45 46	- 97.745 1	213.05 19	- 98.087 5	151.48 63	- 95.86 6
1ST	C34	67	1.5(DL+ EQXP)	1.45	173.63 97	- 94.201 7	164.93 45	- 82.242 1	234.45 46	- 97.745 1	213.05 19	- 98.087 5	151.48 63	- 95.86 6
1ST	C34	67	1.5(DL+ EQXP)	2.9	173.63 97	- 94.201 7	164.93 45	- 82.242 1	234.45 46	- 97.745 1	213.05 19	- 98.087 5	151.48 63	- 95.86 6
1ST	C34	67	1.5(DL+ EQYP)	0	23.617 3	411.90 01	- 55.946 9	175.33 19	- 73.110 5	244.23 16	- 69.111 7	246.12 96	- 10.519 7	278.2 746
1ST	C34	67	1.5(DL+ EQYP)	1.45	23.617 3	411.90 01	- 55.946 9	175.33 19	- 73.110 5	244.23 16	- 69.111 7	246.12 96	- 10.519 7	278.2 746
1ST	C34	67	1.5(DL+ EQYP)	2.9	23.617 3	411.90 01	- 55.946 9	175.33 19	- 73.110 5	244.23 16	- 69.111 7	246.12 96	- 10.519 7	278.2 746
					C	Column Sh	ear, V in Ha	ard Soil						
1ST	C34	67	1.5(DL+ EQXP)	0	205.54 72	- 106.90 31	201.49 95	- 92.289 9	288.66 91	- 111.33 02	261.36 13	- 111.74 82	182.00 27	- 108.9 974
1ST	C34	67	1.5(DL+ EQXP)	1.45	205.54 72	- 106.90 31	201.49 95	- 92.289 9	288.66 91	- 111.33 02	261.36 13	- 111.74 82	182.00 27	- 108.9 974
1ST	C34	67	1.5(DL+ EQXP)	2.9	205.54 72	- 106.90 31	201.49 95	- 92.289 9	288.66 91	- 111.33 02	261.36 13	- 111.74 82	182.00 27	- 108.9 974
1ST	C34	67	1.5(DL+ EQYP)	0	21.328 5	514.56 01	- 69.729 9	223.99 57	- 89.002 7	308.59 71	- 85.119	310.93 01	- 16.931 2	350.4 252
1ST	C34	67	1.5(DL+ EQYP)	1.45	21.328 5	514.56 01	- 69.729 9	223.99 57	- 89.002 7	308.59 71	- 85.119	310.93 01	- 16.931 2	350.4 252
1ST	C34	67	1.5(DL+ EQYP)	2.9	21.328 5	514.56 01	- 69.729	223.99 57	- 89.002 7	308.59 71	- 85.119	310.93 01	- 16.931 2	350.4 252

Table 8: Column Torsion, T for structures with the load combination 1.5 (DL+EQXP) & 1.5 (DL+EQYP), All value in "kN-m"

	(Column Torsi	ion, T in Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"	"T"	"T"	"T"
1ST	C34	67	1.5(DL+EQXP)	0	-52.0172	-36.6355	-56.0881	-52.909	-54.7871
1ST	C34	67	1.5(DL+EQXP)	1.45	-52.0172	-36.6355	-56.0881	-52.909	-54.7871
1ST	C34	67	1.5(DL+EQXP)	2.9	-52.0172	-36.6355	-56.0881	-52.909	-54.7871
1ST	C34	67	1.5(DL+EQYP)	0	56.6478	39.9718	61.1286	57.7035	60.7221
1ST	C34	67	1.5(DL+EQYP)	1.45	56.6478	39.9718	61.1286	57.7035	60.7221
1ST	C34	67	1.5(DL+EQYP)	2.9	56.6478	39.9718	61.1286	57.7035	60.7221
				Column To	rsion, T in Mediur	n Soil			
1ST	C34	67	1.5(DL+EQXP)	0	-70.743	-49.7861	-76.2378	-76.3444	-74.4626
1ST	C34	67	1.5(DL+EQXP)	1.45	-70.743	-49.7861	-76.2378	-76.3444	-74.4626
1ST	C34	67	1.5(DL+EQXP)	2.9	-70.743	-49.7861	-76.2378	-76.3444	-74.4626
1ST	C34	67	1.5(DL+EQYP)	0	77.0414	54.3999	83.1769	83.3566	82.6299
1ST	C34	67	1.5(DL+EQYP)	1.45	77.0414	54.3999	83.1769	83.3566	82.6299
1ST	C34	67	1.5(DL+EQYP)	2.9	77.0414	54.3999	83.1769	83.3566	82.6299
				Column T	orsion, T in Hard	Soil			
1ST	C34	67	1.5(DL+EQXP)	0	-86.8679	-61.1102	-93.589	-93.7234	-91.4055
1ST	C34	67	1.5(DL+EQXP)	1.45	-86.8679	-61.1102	-93.589	-93.7234	-91.4055
1ST	C34	67	1.5(DL+EQXP)	2.9	-86.8679	-61.1102	-93.589	-93.7234	-91.4055
1ST	C34	67	1.5(DL+EQYP)	0	94.6026	66.824	102.1629	102.3801	101.4949
1ST	C34	67	1.5(DL+EQYP)	1.45	94.6026	66.824	102.1629	102.3801	101.4949
1ST	C34	67	1.5(DL+EQYP)	2.9	94.6026	66.824	102.1629	102.3801	101.4949

Table 9: Column Axial Force, P for structures with the load Cases EQXP & EQYP, All value in "kN"

	Co	lumn Axial F	orce, P in Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"	"P"	"P"	"P"
1ST	C34	67	EQXP	0	-1774.1609	-1935.2327	-2180.0176	-1997.9011	-1950.1714
1ST	C34	67	EQXP	1.45	-1774.1609	-1935.2327	-2180.0176	-1997.9011	-1950.1714
1ST	C34	67	EQXP	2.9	-1774.1609	-1935.2327	-2180.0176	-1997.9011	-1950.1714
1ST	C34	67	EQYP	0	-1323.8079	-1094.5008	-1194.8361	-1134.2601	-1152.661
1ST	C34	67	EQYP	1.45	-1323.8079	-1094.5008	-1194.8361	-1134.2601	-1152.661
1ST	1ST C34 67 EQYP 2.9		2.9	-1323.8079	-1094.5008	-1194.8361	-1134.2601	-1152.661	
				Column Axial	Force, P in Mediu	um Soil			
1ST	C34	67	EQXP	0	-2412.8589	-2631.9165	-2964.824	-2885.2394	-2652.2331
1ST	C34	67	EQXP	1.45	-2412.8589	-2631.9165	-2964.824	-2885.2394	-2652.2331
1ST	C34	67	EQXP	2.9	-2412.8589	-2631.9165	-2964.824	-2885.2394	-2652.2331
1ST	C34	67	EQYP	0	-1800.3788	-1488.5211	-1624.9771	-1637.9951	-1567.6189
1ST	C34	67	EQYP	1.45	-1800.3788	-1488.5211	-1624.9771	-1637.9951	-1567.6189
1ST	C34	67	EQYP	2.9	-1800.3788	-1488.5211	-1624.9771	-1637.9951	-1567.6189
				Column Axia	al Force, P in Har	d Soil			
1ST	C34	67	EQXP	0	-2962.8488	-3231.8386	-3640.6295	-3542.9042	-3256.7862
1ST	C34	67	EQXP	1.45	-2962.8488	-3231.8386	-3640.6295	-3542.9042	-3256.7862
1ST	C34	67	EQXP	2.9	-2962.8488	-3231.8386	-3640.6295	-3542.9042	-3256.7862
1ST	C34	67	EQYP	0	-2210.7593	-1827.8164	-1995.3763	-2011.3616	-1924.9438
1ST	C34	67	EQYP	1.45	-2210.7593	-1827.8164	-1995.3763	-2011.3616	-1924.9438
IST C34 67 EQYP 2.9				2.9	-2210.7593	-1827.8164	-1995.3763	-2011.3616	-1924.9438

Table 10: Column Moment, M for structures with the load Cases EC	XP & EQYP, All value in "kN-m"
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	Column N	Noment, M	l in Soft So	oil	Struct I	Struct I	Struct II	Struct II	Struct	Struct	Struct	Struct IV	Struct V	Struc t V
"Story"	"Column"	"Unique- Name"	"Load Case- Combo"	"Station"m	"M2"	"М3"	"M2"	"M3"	"M2"	"МЗ"	"M2"	"МЗ"	"M2"	"M3"
1ST	C34	67	EQXP	0	-158.5935	810.312 8	- 104.427 2	927.618 4	- 171.27 25	1242.67 79	- 161.363	1109. 5618	- 168.5 048	826.4 492
1ST	C34	67	EQXP	1.45	-118.987	710.816 2	- 73.0951	813.598 6	- 128.91 04	1073.62 19	- 121.236 7	967.6 594	- 127.5 574	731.2 906
1ST	C34	67	EQXP	2.9	-79.3805	611.319 7	-41.763	699.578 8	- 86.548 4	904.566	- 81.1104	825.7 569	- 86.60 99	636.1 319
1ST	C34	67	EQYP	0	1484.394 1	-26.503	893.976 6	-68.837	1054.1 309	- 86.0128	999.992 5	- 81.43 95	1018. 7804	- 45.70 12
1ST	C34	67	EQYP	1.45	1164.271 4	-19.366	742.229 1	- 25.8578	853.42 12	- 36.4563	809.655 9	- 34.42 38	793.7 946	- 25.70 85
1ST	C34	67	EQYP	2.9	844.1487	-12.229	590.481 7	17.1215	652.71 16	13.1002	619.319 3	12.59 2	568.8 088	- 5.715 8
						Column Mo	ment, M in N	ledium Soil				•		
1ST	C34	67	EQXP	0	- 215.6871	1102.02 54	- 142.020 9	1261.56 1	- 232.93 06	1690.04 19	- 232.975 4	1602.0 199	- 229.16 66	1123 .971
1ST	C34	67	EQXP	1.45	- 161.8223	966.710 1	- 99.4093	1106.49 41	- 175.31 82	1460.12 58	- 175.042 3	1397.1 465	- 173.47 8	994. 5552
1ST	C34	67	EQXP	2.9	- 107.9575	831.394 8	- 56.7977	951.427 1	- 117.70 58	1230.20 98	- 117.109 3	1192.2 73	- 117.78 95	865. 1394
1ST	C34	67	EQYP	0	2018.776	- 36.0441	1215.80 81	- 93.6183	1433.6 18	- 116.977 5	1443.86 61	- 117.58 73	1385.5 413	- 62.1 537
1ST	C34	67	EQYP	1.45	1583.409 1	- 26.3378	1009.43 16	- 35.1666	1160.6 529	- 49.5806	1169.05 6	- 49.702 5	1079.5 607	- 34.9 636
1ST	C34	67	EQYP	2.9	1148.042 2	- 16.6314	803.055 1	23.2852	887.68 78	17.8162	894.245 9	18.182 2	773.58	- 7.77 35
						Column M	oment, M in	Hard Soil						
1ST	C34	67	EQXP	0	- 264.8511	1353.22 23	- 174.393 4	1549.12 27	- 286.02 51	2075.27 21	-286.08	1967.1 862	- 281.40 31	1380 .170 2
1ST	C34	67	EQXP	1.45	- 198.7083	1187.06 31	- 122.068 8	1358.70 96	- 215.28 04	1792.94 86	- 214.941 7	1715.6 137	- 213.02 08	1221 .255 3
1ST	C34	67	EQXP	2.9	- 132.5655	1020.90 39	- 69.7443	1168.29 65	- 144.53 58	1510.62 52	- 143.803 3	1464.0 411	- 144.63 86	1062 .340 3
1ST	C34	67	EQYP	0	2478.938 1	-44.26	1492.94 09	- 114.957 8	1760.3 985	- 143.641 4	1772.98 27	- 144.39 03	1701.3 633	- 76.3 21
1ST	C34	67	EQYP	1.45	1944.333 2	- 32.3412	1239.52 26	- 43.1825	1425.2 135	- 60.8821	1435.53 2	- 61.031 8	1325.6 37	- 42.9 332
1ST	C34	67	EQYP	2.9	1409.728 3	- 20.4224	986.104 4	28.5929	1090.0 284	21.8773	1098.08 13	22.326 7	949.91 07	- 9.54 54

	Colum	nn Shear, V ir	n Soft Soil		Struct I	Struct I	Struct II	Struct II	Struct III	Struct III	Struct IV	Struct IV	Struct V	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case- Combo"	"Station"m	"V2"	"∨3"	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"	"V2"	"V3"
1ST	C34	67	EQXP	0	68.6183	-27.3148	78.6344	-21.6083	116.590 3	-29.2152	97.8638	-27.6733	65.62 67	- 28.2396
1ST	C34	67	EQXP	1.45	68.6183	-27.3148	78.6344	-21.6083	116.590 3	-29.2152	97.8638	-27.6733	65.62 67	- 28.2396
1ST	C34	67	EQXP	2.9	68.6183	-27.3148	78.6344	-21.6083	116.590 3	-29.2152	97.8638	-27.6733	65.62 67	- 28.2396
1ST	C34	67	EQYP	0	-4.9221	220.774 3	-29.6409	104.653 4	-34.1769	138.420 4	-32.4246	131.2666	- 13.78 81	155.162 6
1ST	C34	67	EQYP	1.45	-4.9221	220.774 3	-29.6409	104.653 4	-34.1769	138.420 4	-32.4246	131.2666	- 13.78 81	155.162 6
1ST	C34	67	EQYP	2.9	-4.9221	220.774 3	-29.6409	104.653 4	-34.1769	138.420 4	-32.4246	131.2666	- 13.78 81	155.162 6
Column Shear, V in Medium Soil														
1ST	C34	67	EQXP	0	93.3209	-37.1481	106.942 7	-29.3873	158.562 8	-39.7327	141.292	-39.9538	89.2523	- 38.4059
1ST	C34	67	EQXP	1.45	93.3209	-37.1481	106.942 7	-29.3873	158.562 8	-39.7327	141.292	-39.9538	89.2523	- 38.4059
1ST	C34	67	EQXP	2.9	93.3209	-37.1481	106.942 7	-29.3873	158.562 8	-39.7327	141.292	-39.9538	89.2523	- 38.4059
1ST	C34	67	EQYP	0	-6.694	300.253	-40.3116	142.328 6	-46.4806	188.251 8	-46.8171	189.524 2	-18.7518	211.021 2
1ST	C34	67	EQYP	1.45	-6.694	300.253	-40.3116	142.328 6	-46.4806	188.251 8	-46.8171	189.524 2	-18.7518	211.021 2
1ST	C34	67	EQYP	2.9	-6.694	300.253	-40.3116	142.328 6	-46.4806	188.251 8	-46.8171	189.524 2	-18.7518	211.021 2
						Colum	n Shear, V in	Hard Soil						
1ST	C34	67	EQXP	0	114.592 6	-45.6157	131.319 4	-36.0859	194.705 8	-48.7894	173.498 3	-49.0609	109.596 5	- 47.1602
1ST	C34	67	EQXP	1.45	114.592 6	-45.6157	131.319 4	-36.0859	194.705 8	-48.7894	173.498 3	-49.0609	109.596 5	- 47.1602
1ST	C34	67	EQXP	2.9	114.592 6	-45.6157	131.319 4	-36.0859	194.705 8	-48.7894	173.498 3	-49.0609	109.596 5	- 47.1602
1ST	C34	67	EQYP	0	-8.2199	368.693	-49.5002	174.771 2	-57.0754	231.162 1	-57.4886	232.724 6	-23.0261	259.121 6
1ST	C34	67	EQYP	1.45	-8.2199	368.693	-49.5002	174.771 2	-57.0754	231.162 1	-57.4886	232.724 6	-23.0261	259.121 6
1ST	C34	67	EQYP	2.9	-8.2199	368.693	-49.5002	174.771 2	-57.0754	231.162 1	-57.4886	232.724 6	-23.0261	259.121 6

Table 11: Column Shear, V for structures with the load Cases EQXP & EQYP, All value in "kN"

Table 12: Column Torsion, T for structures with the load Cases EQXP & EQYP, All value in "kN-m"

	(Column Tors	ion, T in Soft Soil		Struct I	Struct II	Struct III	Struct IV	Struct V		
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"	"T"	"T"	"T"		
1ST	C34	67	EQXP	0	-34.6774	-24.353	-37.3143	-35.2051	-36.4362		
1ST	C34	67	EQXP	1.45	-34.6774	-24.353	-37.3143	-35.2051	-36.4362		
1ST	C34	67	EQXP	2.9	-34.6774	-24.353	-37.3143	-35.2051	-36.4362		
1ST	C34	67	EQYP	0	37.766	26.7186	40.8301	38.5365	40.5699		
1ST	C34	67	EQYP	1.45	37.766	26.7186	40.8301	38.5365	40.5699		
1ST	C34	67	EQYP	2.9	37.766	26.7186	40.8301	38.5365	40.5699		
Column Torsion, T in Medium Soil											
1ST	C34	67	EQXP	0	-47.1612	-33.12	-50.7475	-50.8287	-49.5533		
1ST	C34	67	EQXP	1.45	-47.1612	-33.12	-50.7475	-50.8287	-49.5533		
1ST	C34	67	EQXP	2.9	-47.1612	-33.12	-50.7475	-50.8287	-49.5533		
1ST	C34	67	EQYP	0	51.3617	36.3373	55.529	55.6386	55.1751		
1ST	C34	67	EQYP	1.45	51.3617	36.3373	55.529	55.6386	55.1751		
1ST	C34	67	EQYP	2.9	51.3617	36.3373	55.529	55.6386	55.1751		
				Column ⁻	Forsion, T in Hard	Soil					
1ST	C34	67	EQXP	0	-57.9112	-40.6695	-62.315	-62.4147	-60.8485		
1ST	C34	67	EQXP	1.45	-57.9112	-40.6695	-62.315	-62.4147	-60.8485		
1ST	C34	67	EQXP	2.9	-57.9112	-40.6695	-62.315	-62.4147	-60.8485		

1ST	C34	67	EQYP	0	63.0692	44.62	68.1863	68.321	67.7517
1ST	C34	67	EQYP	1.45	63.0692	44.62	68.1863	68.321	67.7517
1ST	C34	67	EQYP	2.9	63.0692	44.62	68.1863	68.321	67.7517

Table 13: Wodal Load Participation Ratios

Modal Load Participation Ratios		Struct I	Struct I	Struct II	Struct II	Struct III	Struct III	Struct IV	Struct IV	Struct V	Struct V	
"Case"	"Item Type"	"Item"	"Static"	"Dynamic"	"Static"	"Dynamic	"Static"	"Dynamic"	"Static"	"Dynamic"	"Static"	"Dynamic"
			%	%	%	%	%	%	%	%	%	%
Modal	Acceleration	UX	99.82	86.71	99.99	94.7	99.98	94.59	99.99	94.54	99.97	91.54
Modal	Acceleration	UY	99.79	87.46	99.98	91.46	99.97	91.85	99.97	91.83	99.97	92.51
Modal	Acceleration	UZ	0	0	0	0	0	0	0	0	0	0



Graph 1: Modal Load Participation Ratios of Structures

Table 14: Modal Periods and Frequencies

		Struct I		St	ruct II	Str	uct III	St	ruct IV	S	truct V
Case	Mode	Period	Frequency	Period	Frequency	Period	Frequency	Period	Frequency	Period	Frequency
		Sec	cyc/sec	Sec	cyc/sec	Sec	cyc/sec	Sec	cyc/sec	Sec	cyc/sec
Modal	1	6.298	0.159	5.785	0.173	6.415	0.156	6.375	0.157	6.382	0.157
Modal	2	6.248	0.16	5.606	0.178	6.32	0.158	6.21	0.161	5.694	0.176
Modal	3	5.545	0.18	4.684	0.213	5.767	0.173	5.792	0.173	5.642	0.177
Modal	4	2.062	0.485	1.701	0.588	2.114	0.473	2.102	0.476	2.088	0.479
Modal	5	1.952	0.512	1.547	0.646	1.958	0.511	1.901	0.526	1.565	0.639
Modal	6	1.603	0.624	1.475	0.678	1.568	0.638	1.575	0.635	1.524	0.656
Modal	7	1.191	0.84	0.9	1.112	1.219	0.82	1.212	0.825	1.19	0.84
Modal	8	1.027	0.974	0.838	1.193	1.028	0.972	0.983	1.017	0.791	1.264
Modal	9	0.803	1.245	0.645	1.551	0.82	1.22	0.815	1.226	0.711	1.406
Modal	10	0.782	1.279	0.613	1.632	0.711	1.406	0.714	1.401	0.703	1.423
Modal	11	0.645	1.55	0.5	2.002	0.641	1.56	0.604	1.656	0.565	1.769
Modal	12	0.581	1.72	0.45	2.222	0.592	1.689	0.589	1.697	0.423	2.363



Graph 2: Modal Periods and Frequencies





Table 15: Compared of column axial forces in soft soil of structures II, III, I	IV, V with structure I
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		Column axia	al forces "P"		Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case- Combo"	"Station"m	"P"	"P"	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	0%	2%	1%	1%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-2%	-1%	-1%	-1%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	1%	2%	1%	1%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-2%	-1%	-1%	-1%
1ST	C34	67	EQXP	0,1.45,2.9	8%	19%	11%	9%
1ST	C34	67	EQYP	0,1.45,2.9	-21%	-11%	-17%	-15%



		Column a	xial forces "P"		Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	1%	2%	2%	1%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-2%	-1%	-1%	-1%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	1%	3%	2%	1%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-2%	-1%	-1%	-1%
1ST	C34	67	EQXP	0,1.45,2.9	8%	19%	16%	9%
1ST	C34	67	EQYP	0,1.45,2.9	-21%	-11%	-10%	-15%

Table 16: Compared of column axial forces in medium soil of structures II, III, IV, V with structure I

Table 17: Compared of column axial forces in hard soil of structures II, III, IV, V with structure I

		Column a	xial forces "P"		Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"P"	"P"	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	1%	3%	2%	1%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-2%	-1%	-1%	-1%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	1%	4%	3%	2%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-3%	-1%	-1%	-2%
1ST	C34	67	EQXP	0,1.45,2.9	8%	19%	16%	9%
1ST	C34	67	EQYP	0,1.45,2.9	-21%	-11%	-10%	-15%

Table 18: Compared of column moment in soft soil of structures II, III, IV, V with structure I

		Column me	oment forces "M"		Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"M"	"M"	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	564%	4%	-5%	4%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-54%	-35%	-42%	-46%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-54%	4%	-1%	4%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-62%	-39%	-47%	-47%
1ST	C34	67	EQXP	0,1.45,2.9	0%	45%	41%	44%
1ST	C34	67	EQYP	0,1.45,2.9	0%	13%	8%	5%

Table 19: Compared of column moment in medium soil of structures II, III, IV, V with structure I

	(Column ma	ment forces "M"		Struct II	Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"M"	"M"	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-187%	5%	5%	5%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-55%	-35%	-34%	-46%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-55%	5%	5%	4%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-62%	-39%	-38%	-47%
1ST	C34	67	EQXP	0,1.45,2.9	0%	45%	45%	44%
1ST	C34	67	EQYP	0,1.45,2.9	0%	13%	13%	5%

		Column ma	oment forces "M"	Struct II	Struct III	Struct IV	Struct V	
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"M"	"M"	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-127%	6%	5%	5%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-55%	-35%	-34%	-46%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-55%	6%	6%	5%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-62%	-39%	-38%	-47%
1ST	C34	67	EQXP	0,1.45,2.9	0%	45%	45%	44%
1ST	C34	67	EQYP	0,1.45,2.9	0%	13%	13%	5%

Table 20: Compared of column moment in hard soil of structures II, III, IV, V with structure I

Table 21: Compared of column shear in soft soil of structures II, III, IV, V with structure I

Column shear forces "V"						Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"V"	"V"	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-17%	14%	1%	-20%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	215%	175%	191%	-599%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-12%	20%	8%	-18%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	166%	148%	155%	955%
1ST	C34	67	EQXP	0,1.45,2.9	13%	41%	30%	-5%
1ST	C34	67	EQYP	0,1.45,2.9	83%	86%	85%	64%

Table 22: Compared of column shear in medium soil of structures II, III, IV, V with structure I

		Column s	shear forces "V"	Struct II	Struct III	Struct IV	Struct V	
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"V"	"V"	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-10%	21%	14%	-17%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	173%	152%	157%	2154%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-5%	26%	18%	-15%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	142%	132%	134%	325%
1ST	C34	67	EQXP	0,1.45,2.9	13%	41%	34%	-5%
1ST	C34	67	EQYP	0,1.45,2.9	83%	86%	86%	64%

Table 23: Compared of column shear in hard soil of structures II, III, IV, V with structure I

		Column s	hear forces "V"	Struct II	Struct III	Struct IV	Struct V	
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"V"	"V"	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-6%	25%	18%	-15%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	154%	140%	143%	521%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-2%	29%	21%	-13%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	131%	124%	125%	226%
1ST	C34	67	EQXP	0,1.45,2.9	13%	41%	34%	-5%
1ST	C34	67	EQYP	0,1.45,2.9	83%	86%	86%	64%



		Column tors	sion forces "T"	Struct II	Struct III	Struct IV	Struct V	
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"	"T"	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-42%	7%	2%	5%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-42%	7%	2%	7%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-42%	7%	2%	5%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-42%	7%	2%	7%
1ST	C34	67	EQXP	0,1.45,2.9	-42%	7%	1%	5%
1ST	C34	67	EQYP	0,1.45,2.9	-41%	8%	2%	7%

Table 24: Compared of column torsion in soft soil of structures II, III, IV, V with structure I

Table 25: Compared of column torsion in medium soil of structures II, III, IV, V with structure I

	Column torsion forces "T"					Struct III	Struct IV	Struct V
"Story"	"Column"	"Unique - Name"	"Load Case-Combo"	"Station"m	"T"	"T"	"T"	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-42%	7%	8%	7%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-42%	7%	8%	7%
1ST	C34	67	EQXP	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	EQYP	0,1.45,2.9	-41%	8%	8%	7%

Table 26: Compared of column torsion in hard soil of structures II, III, IV, V with structure I

		Column t	orsion forces "T"		Struct II	Struct III	Struct IV	Struct V
'Story"	"Column"	"Unique - Name"	"Load Case-Combo"	oad Case-Combo" "Station"m		"T"	"T"	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-42%	7%	8%	7%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-42%	7%	8%	7%
1ST	C34	67	EQXP	0,1.45,2.9	-42%	7%	7%	5%
1ST	C34	67	EQYP	0,1.45,2.9	-41%	8%	8%	7%

Table 27: Compared of column axial forces of medium soil and hard soil with soft soil for Structure -I

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	3%	6%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	2%	4%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	4%	7%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	3%	5%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

4



		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	3%	6%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	2%	4%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	4%	7%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	2%	4%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 28: Compared of column axial forces of medium soil and hard soil with soft soil for Structure -II

Table 29: Compared of column axial forces of medium soil and hard soil with soft soil for Structure -III

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	4%	7%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	2%	4%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	4%	8%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	3%	5%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	4%	7%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	3%	4%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	5%	8%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	3%	5%
1ST	C34	67	EQXP	0,1.45,2.9	31%	44%
1ST	C34	67	EQYP	0,1.45,2.9	31%	44%

Table 30: Compared of column axial forces of medium soil and hard soil with soft soil for Structure -IV

Table 31: Compared of column axial forces of medium soil and hard soil with soft soil for Structure -V

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"P"	"P"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	3%	6%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	2%	4%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	4%	7%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	3%	5%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%



Table 32: Compared of column moment of medium soil and hard soil with soft soil for Structu	ure -I
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		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	20%	32%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	-7%	-14%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	21%	34%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	-11%	-23%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 33: Compared of column moment of medium soil and hard soil with soft soil for Structure -II

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case- Combo"	"Station"m	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	25%	38%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	32%	46%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	26%	39%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	29%	43%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 34: Compared of column moment of medium soil and hard soil with soft soil for Structure -III

	С	"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case- Combo"	"Station"m	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	27%	40%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	27%	41%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	25%	39%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 35: Compared of column moment of medium soil and hard soil with soft soil for Structure -IV

	Co	olumn Mome	"Medium soil"	"Hard soil"		
"Story"	"Column"	"Unique- Name"	"Load Case- Combo"	"Station"m	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	30%	43%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	33%	46%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	31%	43%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	31%	44%
1ST	C34	67	EQXP	0,1.45,2.9	31%	44%
1ST	C34	67	EQYP	0,1.45,2.9	31%	44%

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"M"	"M"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	22%	35%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	415%	169%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	23%	36%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	71%	82%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 36: Compared of column moment of medium soil and hard soil with soft soil for Structure -V

Table 37: Compared of column shear of medium soil and hard soil with soft soil for Structure -I

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	30%	44%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	29%	42%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 38: Compared of column she	ear of medium soil and	hard soil with soft soi	for Structure -II

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case- Combo"	"Station"m	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	56%	61%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	40%	52%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 39: Compared of column shear of medium soil and hard soil with soft soil for Structure -III

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	31%	45%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	29%	43%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

	•					
		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"V"	"٧"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	36%	48%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	31%	43%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	34%	47%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	31%	43%
1ST	C34	67	EQXP	0,1.45,2.9	31%	44%
1ST	C34	67	EQYP	0,1.45,2.9	31%	44%

Table 40: Compared of column shear of medium soil and hard soil with soft soil for Structure -IV

Table 41: Compared of column shear of medium soil and hard soil with soft soil for Structure -V

		"Medium soil"	"Hard soil"			
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"V"	"V"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	30%	44%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	29%	43%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 42: Compared of column torsion of medium soil and hard soil with soft soil for Structure -I

Column Torsion Forces "T"					"Medium soil"	"Hard soil"
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	" T "	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	0%	0%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	26%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 43: Compared of column torsion of medium soil and hard soil with soft soil for Structure -II

Column Torsion Forces "T"					"Medium soil"	"Hard soil"
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"Т"	"Т"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	27%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	27%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Column Torsion Forces "T"					"Medium soil"	"Hard soil"
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"Т"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	27%	40%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	26%	40%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	27%	40%
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%
1ST	C34	67	EQYP	0,1.45,2.9	26%	40%

Table 44: Compared of column torsion of medium soil and hard soil with soft soil for Structure -III

Table 45: Compared of column torsion of medium soil and hard soil with soft soil for Structure -IV

Column Torsion Forces "T"					"Medium soil"	"Hard soil"
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	31%	44%
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	31%	44%
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	31%	44%
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	31%	44%
1ST	C34	67	EQXP	0,1.45,2.9	31%	44%
1ST	C34	67	EQYP	0,1.45,2.9	31%	44%

Column Torsion Forces "T"					"Medium soil"	"Hard soil"	
"Story"	"Column"	"Unique- Name"	"Load Case-Combo"	"Station"m	"T"	"T"	
1ST	C34	67	1.2(DL+LL+EQXP)	0,1.45,2.9	26%	40%	
1ST	C34	67	1.2(DL+LL+EQYP)	0,1.45,2.9	27%	40%	
1ST	C34	67	1.5(DL+EQXP)	0,1.45,2.9	26%	40%	
1ST	C34	67	1.5(DL+EQYP)	0,1.45,2.9	27%	40%	
1ST	C34	67	EQXP	0,1.45,2.9	26%	40%	

0.1.45.2.9

EQYP

Table 46: Compared of column torsion of medium soil and hard soil with soft soil for Structure -V

a) Discussion on Results

1ST

C34

67

When a structure is subjected to earthquake, it responds by vibrating. An example force can be resolved into three mutually perpendicular directionstwo horizontal directions (X and Y directions) and the vertical direction (Z) [8]. This motion causes the structure to vibrate or shake in all three directions; the predominant direction of shaking is horizontal. All the structures are primarily designed for gravity loads-force equal to mass time's gravity in the vertical direction. Vertical acceleration should also be considered in structures with large spans those in which stability for design, or for overall stability analysis of structures. The basic intent of design theory for earthquake resistant structures is that buildings should be able to resist minor earthquakes without damage, resist moderate earthquakes without structural damage but with some non-structural damage. To avoid collapse during a major earthquake, Members must be ductile enough to absorb and dissipate energy by post elastic deformation. Redundancy in the structural system permits redistribution of internal forces in the event of the failure of key elements. When the primary element or system yields or fails, the lateral force can be redistributed to a secondary system to prevent progressive failure.

40%

26%

When a structure is subjected to an earthquake excitation, it interacts with the foundation and the soil, and thus changes the motion of the ground[2,8]. This means that the movement of the whole ground-structure system is influenced by the type of soil as well as by the type of structure. Understanding of soil structure interaction will enable the designer to design structures that will behave better during an earthquake.

V. CONCLUSIONS

From the above results and discussions, following conclusions can be drawn:

- The shear wall and it is position has a significant influenced on the time period, the time period is not influenced by the type of soil, in tall building with box shape Shear Walls is showing the low time period which shows a very significant performance.
- Shear is effected marginally by placing of the shear wall, grouping of shear wall and type of soil. The shear is increased by adding shear wall due to increase the seismic weight of the building.
- The Axial force and Moment in the column increases when the type of soil changes from hard to medium and medium to soft. Since the column moment increase as the soil type changes, soil structure interaction must be suitably considered while designing frames for seismic force.
- It is evident that the maximum column axial force is various with type of soil and placing of the shear wall.
- It is evident that the maximum column shear force in X-direction is influenced by the type of soil and placing of the shear wall.
- It is evident that the maximum column shear force in Y-direction has no influence on the type of soil and placing shear wall.
- It is evident that the maximum column torsion is same for all columns in a structure, but is influenced by the type of soil and placing shear wall.
- It is evident that the maximum column moment in Xdirection has no influence on the type of soil and placing shear wall.
- It is evident that the maximum column moment in Ydirection is influenced by the type of soil and placing of shear wall.
- It is evident that the results from 1.2 (DL + IL \pm EL) combination load is closed to the 1.5 (DL + EL) and there is no more difference between these combination load.
- Based on the analysis and discussion, shear wall are very much suitable for resisting earthquake induced lateral forces in multistoried structural systems when compared to multistoried structural systems whit out shear walls. They can be made to behave in a ductile manner by adopting proper detailing techniques.
- According to IS-1893:2002 the number of modes to be used in the analysis should be such that the total sum of modal masses of all modes considered is at least 90 percent of the total seismic mass. Here the maximum mass is for the tall building with box shape RC shear wall.
- ETABS is the robust software which is utilized foranalyzing any kind of multi building structures.

Acknowledgments

The author would like to express his gratitude to all the individuals for their expertise throughout all aspects of our study and contribution to writing the manuscript. The author would like to express his gratitude to the Nanjing Forestry University, China, for funding this research work through the project No. 163050206 & foreign young talents project No. QN2021014006L. In addition, I thank the anonymous reviewers for their fruitful suggestions to improve the article. The author is truly grateful to all of you.

Conflict of Interest

The author declare no conflict of interest.

Data Availability Statement

All data generated or analysed during this study are included in this article.

References Références Referencias

- 1. *Earthquake Resistant Design of Structures,* Oxford University Press, New Delhi, 2010.
- 2. Dynamics of Structures: Theory and Application to Earthquake Engineering, Pearson Education, 4th edition, New Delhi, 2012.
- Structural Dynamics: Theory & computations, (Second Edition), CBS Publishers & Distributors, New Delhi, 2004.
- 4. *Plain and Reinforced Concrete-Code of practice,* IS 456, New Delhi, India, 2000.
- Ductile detailing of reinforced concrete structures subjected to seismic forces— Code of Practice, IS 13920, New Delhi, India., 1993.
- Dead loads on buildings and Structures, IS 875 (part 1), New Delhi, India, 1987.
- Live loads on buildings and Structures, IS 875 (part 2), New Delhi, India, 1987.
- 8. Criteria for earthquake resistant design of structures General provisions and buildings, IS 1893 (part 1), New Delhi, India, 2002.
- 9. Berkeley, "ETABS Integrated Building Design Software", Computers and Structure, Inc., California, USA, February 2003.
- 10. Makar Nageh, "How to Model and Design High Rise Building Using ETABS Program", scientific book house for publishing and distributing, Cairo, 2007.
- Paulay, T. & Priestley, M. J. N. "Seismic Design of Reinforced Concrete and Masonry Buildings", JOHN WILEY & SONS, INC, New York. 1992.
- 12. J. L. Humar and S. Yavari "Design of concrete shear wall buildings for earthquake induced torsion", *14 structural conference of the Canadian society for civil engineering*, Canada, June, 2002.
- 13. Anand, N., Mightraj, C. and Prince Arulraj, G. "Seismic behaviour of RCC shear wall under different soil conditions", *Indian geotechnical conference*, pp 119-120, december 2010.

- Gaikwad Ujwala Vithal, "Effect of Shear Wall on Seismic Behavior of Unsymmetrical Reinforced Concrete Structure", International Journal of Research and Scientific Innovation (IJRSI) Volume IV, Issue X, October 2017.
- 15. Mahantesh S Patil & R B Khadiranaikar, "Dynamic Analysis of High Rise RC Structure with Shear Walls and Coupled Shear Walls", International Journal of Advance Engineering and Research Development, Volume 2, Issue 8, August, 2015.
- 16. Durgesh C. Rai, Sudhir K. Jain and C. V. R. Murty, "Seismic Design of RC Structures", short course, conducted by Department of Civil Engineering, IIT Kanpur, Ahmedabad, India, Nov 25-30, 2012.